DESIGN CRITERIA:
(2006 INTERNATIONAL BUILDING CODE W/ 2007 GEORGIA AMENDMENTS)

1. ROOF SNOW LOAD:
   A. GROUND SNOW LOAD (Pg) = 5 psf
2. DESIGN ROOF AND FLOOR LOADS:
   A. ROOF DEAD LOAD = 20 psf (SEE TRUSS NOTES FOR ADD'L INFORMATION)
   B. ROOF LIVE LOAD = 20 psf (REDUCIBLE PER ROOF SLOPE AND TRIBUTARY AREA)
   C. FLOOR DEAD LOAD = 20 psf (WOOD), 42 psf (STEEL/CONCRETE)
   D. FLOOR LIVE LOAD = 40 psf (CLASSROOMS)
      = 80 psf (CORRIDORS ABOVE FIRST FLOOR)
      = 100 psf (ASSEMBLY AREAS)
3. WIND LOADS:
   A. BASIC WIND SPEED (V) = 90 mph (3 SECOND GUST)
   B. WIND LOAD IMPORTANCE (I_w) = 1.15 (OCCUPANCY CATEGORY III)
   C. EXPOSURE B
   D. INTERNAL PRESSURE COEFFICIENT (GC_{pi}) = ±0.18
   E. REFER TO THE DIAGRAMS ON THIS SHEET FOR WIND LOADS TO COMPONENT AND CLADDING ELEMENTS DESIGNED BY OTHERS.
   F. DESIGN STEEL ROOF JOISTS FOR A NET UPLIFT EQUAL TO THE UPLIFT PRESSURE GIVEN IN THE COMPONENTS AND CLADDING DIAGRAM MINUS 10 PSF DEAD LOAD.
4. SEISMIC DESIGN DATA:
   A. SEISMIC IMPORTANCE (I_e) = 1.25 (OCCUPANCY CATEGORY III)
   B. MAPPED SPECTRAL RESPONSE ACCELERATIONS S_S = 0.267, S_1 = 0.092
   C. SITE CLASS D
   D. SPECTRAL RESPONSE COEFFICIENTS S_{DS} = 0.282, S_{D1} = 0.148
   E. SEISMIC DESIGN CATEGORY C
   F. SEISMIC-FORCE-RESISTING SYSTEM: LIGHT-FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE (BEARING WALL SYSTEM)
   G. DESIGN BASE SHEAR = 23.7 KIPS
   H. SEISMIC RESPONSE COEFFICIENT, C_s = 0.054
   I. RESPONSE MODIFICATION FACTOR, R = 6 1/2
   J. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
5. DESIGN LOAD COMBINATIONS (ALLOWABLE STRESS DESIGN):
   D + L
   D + (Lr or S or R)
   D + 3/4L + 3/4(Lr or S or R)
   D + (W or 0.7E)
   D + 3/4(W or 0.7E) + 3/4L + 3/4(Lr or S or R)
   0.6D + W
   0.6D + 0.7E

   REFER TO ASCE 7-05 CHAPTER 12.4 FOR ADDITIONAL SEISMIC LOAD EFFECTS AND COMBINATION REQUIREMENTS.

   INCREASES IN ALLOWABLE STRESSES ARE NOT PERMITTED WITH THESE LOAD COMBINATIONS.

   COMBINATIONS THAT CONTAIN SNOW LOAD (S) SHALL INCLUDE EFFECTS OF SNOW DRIFTING WHERE APPLICABLE.

   ONLY 20% OF FLAT ROOF SNOW LOADS IN EXCESS OF 30 psf MUST BE COMBINED WITH SEISMIC LOADS.

FOUNDATIONS – GENERAL:

1. THE FOUNDATION HAS BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS MADE IN THE GEOTECHNICAL REPORT PREPARED BY CONSTRUCTION MATERIALS SERVICES, INC., DATED AUGUST 12, 2008.
2. CONTINUOUS WALL FOOTINGS & SPREAD FOOTINGS SHALL BEAR ON A SOIL CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF 2,500 psf UNDER SERVICE LIVE AND DEAD LOADS.
3. FOOTINGS MAY BE POURED INTO AN EARTH-FORMED TRENCH IF SOIL CONDITIONS PERMIT.
4. ALL BEARING MATERIAL SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
5. BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 1'-0" BELOW FINAL FINISHED GRADES.
6. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.
7. WHERE FOUNDATION WALLS ARE TO HAVE EARTH PLACED ON EACH SIDE, PLACE FILL SIMULTANEOUSLY SO AS TO MAINTAIN A COMMON ELEVATION ON EACH SIDE OF THE WALL.
8. FOUNDATION CONCRETE SHALL HAVE REACHED A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI BEFORE BEING LOADED. STRENGTHS SHALL BE VERIFIED BY TESTS.

CONCRETE:

1. CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH (f'c) OF 3,000 psi AND A DENSITY OF 145 pcf, UNLESS NOTED OTHERWISE.
2. REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. (SYNTHETIC FIBER REINFORCING MAY BE USED INSTEAD OF WELDED WIRE FABRIC FOR SLABS-ON-GROUND WHERE INDICATED. REFER TO S1.0 NOTE 2 FOR MORE INFO.)
4. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:
   UNFORMED SURFACE IN CONTACT WITH THE GROUND. ——— 3 IN.
   FORMED SURFACES EXPOSED TO EARTH OR WEATHER:
   #6 BARS AND LARGER ——— 2 IN.
   #5 BARS AND SMALLER ——— 1 1/2 IN.
   FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:
   BEAMS, GIRDERS, AND COLUMNS ——— 1 1/2 IN.
   SLABS, WALLS, AND JOISTS:
   #11 BARS AND SMALLER ——— 3/4 IN.
   #14 AND #18 BARS ——— 1 1/2 IN.
5. LAP SPLICES SHALL BE IN ACCORDANCE WITH THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES.

BAR SIZE	TENSION SPLICES (INCHES) TOP BARS A		OTHER BARS A		COMPRESSION SPLICES (INCHES) ALL BARS
	A	B	A	B	
#3	22	28	17	22	12
#4	29	37	22	29	15
#5	36	47	28	36	19
#6	43	56	33	43	23
#7	63	81	48	63	27
#8	72	93	55	72	30
#9	81	105	62	81	34
#10	91	116	70	91	38
#11	101	131	76	101	42

COMPRESSION DOWEL EMBEDMENT: 22 BAR DIAMETERS
LAP WELDED WIRE FABRIC ONE SPACING OF CROSS WIRES PLUS 2".

6. BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" OF CONCRETE.

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO THE FOLLOWING GRADES:
   ALL WIDE FLANGES (U.N.O.) ——— A992 (Fy = 50 ksi)
   ALL CHANNELS, ANGLES, PLATES, ETC. ——— A36 (Fy = 36 ksi)
   STRUCTURAL TUBE ——— A500 (Fy = 46 ksi) (GRADE B)
   STEEL PIPE ——— A53 (Fy = 35 ksi)
   ANCHOR BOLTS ——— A307, A36, OR F1554
   BOLTS ——— A325
   WELDING ELECTRODES ——— E70XX
2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE (2005), EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
3. THE STEEL STRUCTURE IS A NON-SELF-SUPPORTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE FLOOR AND/OR ROOF SYSTEMS AND ATTACHMENT TO THE PERIMETER SHEAR WALLS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF PROVIDING THIS SUPPORT.
4. THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. CONNECTION DETAILS INDICATED ON THE DRAWINGS SHALL BE INCORPORATED INTO FABRICATOR'S CONNECTION DESIGN. SEE SPECIFICATIONS. ALL SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE FABRICATOR'S ENGINEER WITH THE ENGINEER'S SEAL FOR THE STATE WHERE THE STRUCTURE IS LOCATED. ENGINEER'S SEAL MAY BE QUALIFIED "FOR DESIGN OF CONNECTIONS ONLY". UNLESS DESIGN END REACTIONS ARE SPECIFICALLY SHOWN OTHERWISE IN THE DESIGN DRAWINGS, BEAM END CONNECTIONS SHALL BE CAPABLE OF SUPPORTING ONE HALF OF THE MAXIMUM TOTAL UNIFORM LOAD LISTED IN PART 3 OF AISC's "MANUAL OF STEEL CONSTRUCTION", 13th EDITION, FOR THE BEAM SIZE AND YIELD STRENGTH UTILIZED.
5. REVIEW OF SHOP DRAWINGS DOES NOT RELIEVE THE FABRICATOR OF RESPONSIBILTY FOR THE ADEQUACY OF ALL CONNECTIONS.
6. CONTRACTOR SHALL REMOVE ALL MUD OR OTHER UNSIGHTLY DELETERIOUS MATERIAL FROM STRUCTURAL STEEL AFTER ERECTION OF STEEL IS COMPLETE. ALL FIELD WELDING SHALL BE TOUCHED UP WITH PRIMER PAINT UPON COMPLETION.

MISCELLANEOUS:

1. THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
2. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
3. NO OPENINGS SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
4. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
5. OPENINGS 1'-4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
6. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
7. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
8. THE CONTRACT DRAWINGS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SEQUENCES, AND SAFETY. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR:
   A. CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES.
   B. PRECAUTIONS AND PROGRAMS FOR SAFETY IN CONNECTION WITH THE WORK.
   C. THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
9. DO NOT SCALE THESE DRAWINGS, USE DIMENSIONS SHOWN.
10. CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
11. THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE PROFESSIONAL OF RECORD HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
12. WHERE REQUIRED, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW BY THE STRUCTURAL ENGINEER. FABRICATION OF MATERIAL SHALL NOT BEGIN PRIOR TO RETURN OF SHOP DRAWINGS MARKED "NO EXCEPTIONS TAKEN" OR "EXCEPTIONS NOTED". WHERE EXCEPTIONS ARE NOTED, FIELD USE DRAWINGS SHALL BE SUBMITTED SHOWING THE REQUIRED CORRECTIONS. REFER TO THE PROJECT SPECIFICATIONS FOR REQUIRED SHOP DRAWING SUBMITTALS.
13. THE CONTRACTOR SHALL NOT IMPLEMENT REMEDIAL DETAILS NECESSITATED BY MISFABRICATION OR FAULTY CONSTRUCTION WITHOUT WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER.

WOOD CONNECTIONS:

1. SEE FASTENER SCHEDULE ON THIS SHEET FOR TYPICAL FASTENING REQUIREMENTS NOT SPECIFICALLY SHOWN ON THESE PLANS.
2. MAKE FRAMED CONNECTIONS WITH APPROVED FRAMING ANCHORS ON EACH SIDE OR APPROVED JOIST HANGERS BY SIMPSON OR APPROVED EQUAL.
3. PRE-DRILL ALL HOLES FOR NAILS LARGER THAN 20d & ALL LAG SCREWS.
4. FIELD DRILL BOLT HOLES FOR PROPER MATCHING AND BEARINGS.
5. PROVIDE CUT WASHERS AT BOLTS IN WOOD WITHOUT STEEL PLATES.
6. ALL FRAMING ANCHORS & FASTENERS USED w/ P.T. WOOD SHALL BE HOT DIPPED GALVANIZED.

REINFORCED MASONRY:

1. REINFORCED MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH, f'm, OF 1500 PSI. MASONRY UNITS SHALL BE NORMAL WEIGHT BLOCK CONFORMING TO ASTM C90, GRADE N, TYPE 1, AND SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2000 PSI. MORTAR SHALL CONFORM TO ASTM C270, TYPE S. GROUT SHALL CONFORM TO ASTM C476.
2. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
3. CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED TRUSS OR LADDER TYPE FORMED FROM 9 GAUGE COLD-DRAWN STEEL WIRE COMPLYING WITH ASTM A82. JOINT REINFORCING SHALL BE SPACED AT 16" O.C. VERTICALLY IN ALL MASONRY WALLS.
4. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF VERTICAL CONTROL JOINTS. HORIZONTAL BOND BEAM AND LINTEL REINFORCING SHALL BE CONTINUOUS ACROSS VERTICAL CONTROL JOINTS. JOINT REINFORCING SHALL BE STOPPED EITHER SIDE OF VERTICAL CONTROL JOINTS.
5. ALL REINFORCED CELLS, ALL CELLS BELOW GRADE, AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
6. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN 6 VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
7. REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
8. MINIMUM TENSION LAP SPLICE LENGTHS FOR REINFORCING SHALL BE 25" FOR #4 BARS, 31" FOR #5 BARS, AND 57" FOR #6 BARS. SPLICED BARS SHALL BE WIRED TOGETHER.
9. VERTICAL BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING, NOR 10 FEET. BARS SHALL BE IN PLACE PRIOR TO GROUTING.
10. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4 OF AN INCH FROM THE MASONRY AND NOT LESS THAN ONE BAR DIAMETER BETWEEN BARS.
11. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 3"x4".
12. GROUTING SHALL BE STOPPED 1-1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT.
13. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
14. ALL BOLTS, ANCHORS, ETC., INSERTED IN THE WALLS, SHALL BE GROUTED SOLID INTO POSITION, UNLESS NOTED OTHERWISE.

SPECIAL INSPECTIONS:

1. SPECIAL INSPECTIONS SHALL BE PROVIDED DURING CONSTRUCTION FOR THE FOLLOWING:
   A. CONCRETE:
      PROVIDE INSPECTIONS DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE (EXCEPT SLABS ON GRADE).
   B. BOLTS INSTALLED IN CONCRETE:
      PROVIDE INSPECTIONS PRIOR TO AND DURING THE PLACEMENT OF CONCRETE AROUND BOLTS IDENTIFIED ON THE DRAWINGS (IF ANY) AS REQUIRING SPECIAL INSPECTIONS.
   C. REINFORCING STEEL:
      PROVIDE INSPECTIONS FOR CONFORMANCE WITH THE APPROVED PLANS PRIOR TO CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOB SITE.
   D. STRUCTURAL WELDING:
      PROVIDE PERIODIC INSPECTIONS OF WORK IN PROGRESS AND A VISUAL INSPECTION OF ALL WELDS (INCLUDING WELDING OF METAL ROOF DECK) PRIOR TO COMPLETION OF WORK. INSPECTION IS NOT REQUIRED FOR WELDING DONE IN AN APPROVED FABRICATOR'S SHOP.
   E. HIGH STRENGTH BOLTING:
      PROVIDE PERIODIC INSPECTIONS TO ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO SNUG CONTACT.
   F. STRUCTURAL MASONRY:
      PROVIDE PERIODIC INSPECTIONS DURING PREPARATION AND TAKING OF ANY REQUIRED PRISMS OR TEST SPECIMENS, AT THE START OF LAYING UNITS, AFTER THE LAYING OF REINFORCING STEEL, GROUT SPACE PRIOR TO EACH GROUTING OPERATION, AND DURING ALL GROUTING OPERATIONS.
2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
   A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.
   B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL OF RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, SUBMIT A COMPLETE LIST OF ALL OUTSTANDING DISCREPANCIES ON A WEEKLY BASIS TO THE OWNER, THE BUILDING OFFICIAL, AND THE PROFESSIONAL OF RECORD UNTIL ALL CORRECTIONS HAVE BEEN COMPLETED.
4. WHERE SPECIAL INSPECTION REQUIREMENTS DUPLICATE THE REQUIREMENTS OF SPECIFIED QUALITY ASSURANCE TESTING, DUPLICATE INSPECTIONS SHALL NOT BE REQUIRED.

PRE-ENGINEERED WOOD TRUSSES

1. WOOD TRUSSES SHALL BE AS SPECIFIED BY THE DRAWINGS AND SHALL BE DESIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF GEORGIA. DESIGN SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF THE INTERNATIONAL BUILDING CODE WITH GEORGIA AMENDMENTS. ALL TRUSSES SHALL BE CAPABLE OF SUSTAINING ALL LOADS SPECIFIED HEREIN.
2. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW BY THE STRUCTURAL ENGINEER. SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE PROJECT STRUCTURAL ENGINEER OF RECORD.
3. ALL HARDWARE (BOLTS, HANGERS, STRAPS, ETC.) REQUIRED FOR CONNECTIONS BETWEEN PRE-ENGINEERED TRUSSES SHALL BE DESIGNED AND SPECIFIED BY THE TRUSS DESIGN ENGINEER.
4. WOOD ROOF TRUSSES SHALL BE DESIGNED FOR A UNIFORM DEAD LOAD OF 10 psf, APPLIED TO BOTH THE TOP AND BOTTOM CHORDS (20 psf TOTAL). ROOF TRUSS TOP CHORDS SHALL BE DESIGNED FOR A UNIFORM LIVE LOAD OF 20 psf. TOP CHORD DEAD LOAD DOES NOT INCLUDE ALLOWANCE FOR MANSARD TRUSSES. ACTUAL WEIGHT OF MANSARD TRUSSES IS TO BE APPLIED IN ADDITION TO THE UNIFORM TOP CHORD DEAD LOAD INDICATED. ALSO, REFER TO THE ROOF FRAMING PLAN FOR ROOF TOP EQUIPMENT LOCATIONS AND WEIGHTS. THE UNIT WEIGHTS ARE NOT INCLUDED IN THE UNIFORM TOP CHORD DEAD LOAD.
5. WOOD FLOOR TRUSSES SHALL BE DESIGNED FOR A UNIFORM DEAD LOAD OF 15 psf APPLIED TO THE TOP CHORDS AND 5 psf APPLIED TO THE BOTTOM CHORDS (20 psf TOTAL). WOOD TRUSS TOP CHORDS SHALL BE DESIGNED FOR A UNIFORM LIVE LOAD OF 40 psf IN CLASSROOM AREAS AND 80 PSF IN CORRIDORS.
6. ROOF TRUSS DEFLECTION CRITERA: L/180 UNDER TOTAL DEAD PLUS LIVE LOAD, L/240 UNDER LIVE OR WIND LOAD. FLOOR TRUSS DEFLECTION CRITERIA: L/240 UNDER TOTAL DEAD PLUS LIVE LOAD, L/360 UNDER LIVE LOAD. IN EACH CASE, L IS THE SPAN LENGTH IN INCHES.
7. TRUSSES SHALL BE PROVIDED WITH ALL BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION STABILITY AND FOR STABILITY AGAINST WIND AND/OR SEISMIC LOADS AS REQUIRED BY DESIGN. GABLE END TRUSSES SHALL BE PROVIDED WITH ADDITIONAL WEB MEMBERS AND/OR DIAGONAL BRIDGING TO BRACE AGAINST WIND LOADS ACTING PERPENDICULAR TO THE FACE OF THE TRUSS.

GENERAL WOOD INFORMATION:

1. EACH PIECE OF LUMBER SHALL BE "S-DRY" AND BEAR THE STAMP OF A GRADING RULES AGENCY APPROVED BY THE AMERICAN LUMBER STANDARDS COMMITTEE (ALSC).
2. EACH PIECE OF LUMBER IN PLACE IN THE STRUCTURE SHALL BE OF THE ORIGINAL GRADE SPECIFIED OR BETTER WHEN INSPECTED BY A GRADING AGENCY APPROVED BY THE ALSC, REGARDLESS OF REQUIRED GRADE STAMP AND CERTIFICATIONS.
3. DO NOT NOTCH OR DRILL JOISTS, BEAMS OR LOAD BEARING STUDS WITHOUT APPROVAL.
4. PROVIDE METAL OR 1 x 3 WOOD CROSS BRIDGING OR SOLID BLOCKING AT 8'-0" O.C. (MAX).
5. PROVIDE 2" SOLID BLOCKING AT JOIST SUPPORTS.
6. ALL WOOD IN CONTACT WITH EARTH, CONCRETE, OR WEATHER SHALL BE PRESSURE TREATED (P.T.).
7. ALL STRUCTURAL SAWN LUMBER SHALL BE SPRUCE-PINE-FIR, No. 1/No. 2, UNLESS NOTED OTHERWISE ON PLANS, DETAILS, OR NOTES.
8. ALL LUMBER SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19%.

TYPICAL WOOD FRAMING FASTENING SCHEDULE

TYPE OF CONNECTION	FASTENER TYPE & SPACING		LOCATION OF & TYPE OF FASTENING
SOLE PLATE TO JOIST OR BLOCKING	16d NAILS @ 16" O.C. (3) 3" x 0.131" NAILS @ 8" O.C. (3) 3" 14 GAGE STAPLES @ 12" O.C.		TYPICAL FACE NAIL
TOP PLATE TO STUD	(2) 16d COMMON NAILS (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES		END NAIL
STUD TO SOLE PLATE	(4) 8d COMMON NAILS (4) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES		TOENAIL
	(2) 16d COMMON NAILS (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES		END NAIL
DOUBLE STUDS	16d NAILS @ 24" O.C. (3) 3" x 0.131" NAILS @ 8" O.C. (3) 3" 14 GAGE STAPLES @ 12" O.C.		FACE NAIL
DOUBLE TOP PLATES	16d NAILS @ 16" O.C. (3) 3" x 0.131" NAILS @ 8" O.C. (3) 3" 14 GAGE STAPLES @ 12" O.C.		TYPICAL FACE NAIL
	(8) 16d COMMON NAILS (12) 3" x 0.131" NAILS (12) 3" 14 GAGE STAPLES TYPICAL FACE NAIL		LAP SPLICE
RIM JOIST TO TOP PLATE	8d COMMON NAILS AT 6" O.C. 3" x 0.131" NAILS AT 6" O.C. 3" 14 GAGE STAPLES AT 6" O.C.		TOENAIL
TOP PLATES, LAPS & INTERSECTIONS	(2) 16d COMMON NAILS (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES		FACE NAIL
CONTINUOUS HEADER, TWO PIECES	16d COMMON NAILS		16" O.C. ALONG EDGE
CONTINUOUS HEADER TO STUD	(4) 8d COMMON NAILS		TOENAIL
BUILT-UP CORNER STUDS	16d COMMON NAILS 3" x 0.131" NAILS 3" 14 GAGE STAPLES		@ 24" O.C. @ 16" O.C. @ 16" O.C.
BUILT-UP GIRDERS & BEAMS	20d COMMON NAILS @ 32" O.C. 3" x 0.131" NAILS @ 24" O.C. 3" 14 GAGE STAPLES @ 24" O.C.		FACE NAIL AT TOP & BOTTOM STAGGERED ON OPPOSITE SIDES.
	(2) 20d COMMON NAILS (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES		FACE NAIL AT ENDS & AT EACH SPLICE.
ROOF RAFTER OR TRUSS TO TOP PLATE OF WALL, BEAM, OR NAILER	SIMPSON H1 UPLIFT ANCHOR @ EA. BEARING LOCATION		(6) 8d x 1 1/2" TO RAFTER/TRUSS (4) 8d TO PLATES
ROOF SHEATHING ATTACHMENT	5/8" SHEATHING	10d COMMON NAILS @ 6" O.C. ALONG PANEL EDGES & @ 12" O.C. ALONG INTERMEDIATE SUPPORTS	FASTEN AT ALL INTERMEDIATE SUPPORTS AND PANEL EDGES AS SCHEDULED, STAGGER JOINTS
WALL SHEATHING ATTACHMENT	1/2" SHEATHING	10d COMMON NAILS @ 4" O.C. ALONG PANEL EDGES & @ 12" O.C. ALONG INTERMEDIATE SUPPORTS	AT ALL SUPPORTED EDGES AND INTERMEDIATE SUPPORTS
FLOOR SHEATHING ATTACHMENT	3/4" SHEATHING	GLUE TO ALL SUPPORTING MEMBERS AND SCREW w/ 2½" No. 8 SCREWS @ 6" O.C. ALONG PANEL EDGES & @ 12" O.C. ALONG INTERMEDIATE SUPPORTS	AT ALL SUPPORTED EDGES AND INTERMEDIATE SUPPORTS

TIMBER HEADER/LINTEL SCHEDULE

LINTEL MARK	LINTEL SIZE	JAMB FRAMING	COMMENTS
L1	LVL 5.25x9.25	(3)-JACK STUD & (3)-FULL HEIGHT STUDS EA. SIDE	NOTES 2, 3, & 4
L2	LVL 5.25x11.25	(3)-JACK STUD & (3)-FULL HEIGHT STUDS EA. SIDE	NOTES 2, 3, & 4
L3	(3) 2x8 w/(2) 1/2" PLYWOOD SPACERS	(2)-JACK STUDS & (2)-FULL HEIGHT STUDS EA. SIDE	NOTES 1-3
L4	LVL 3.5x7.25 1/2" PLYWOOD SPACERS	(3)-JACK STUD & (2)-FULL HEIGHT STUDS EA. SIDE	NOTES 2, 3, & 4
L5	HSS 7x5x3/8"	STEEL COLUMN, SEE COL. SCHEDULE	

NOTES:
1. SPRUCE-PINE-FIR No.1/No.2, SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.
2. JACK STUD & FULL HEIGHT STUDS TO BE MECHANICALLY LAMINATED PER DETAIL 9/S2.3.
3. ALL JAMB FRAMING TO BE SPRUCE-PINE-FIR No.1/No.2.
4. SEE PLAN NOTES ON S1.1 FOR LAMINATED VENEER LUMBER (LVL) INFORMATION.
5. PROVIDE (2) 2x6 W/ 1/2" PLYWOOD SPACER AT DOOR OPENINGS IN NON-LOAD BEARING INTERIOR 2x4 WALLS (W/ (1) JACK STUD AND FULL HEIGHT STUD EACH SIDE).

PROGRESS SET - NOT FOR CONSTRUCTION

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NOT RELEASED FOR CONSTRUCTION

PROFESSIONAL SEAL

PROJECT LOCATION
3034 OLD ATLANTA ROAD
CUMMING, GA 30041

MONTESSORI SCHOOL

REVISIONS

GENERAL NOTES

DATE: 7-15-09
PROJECT No.: 29001

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SHEET No. S0.1